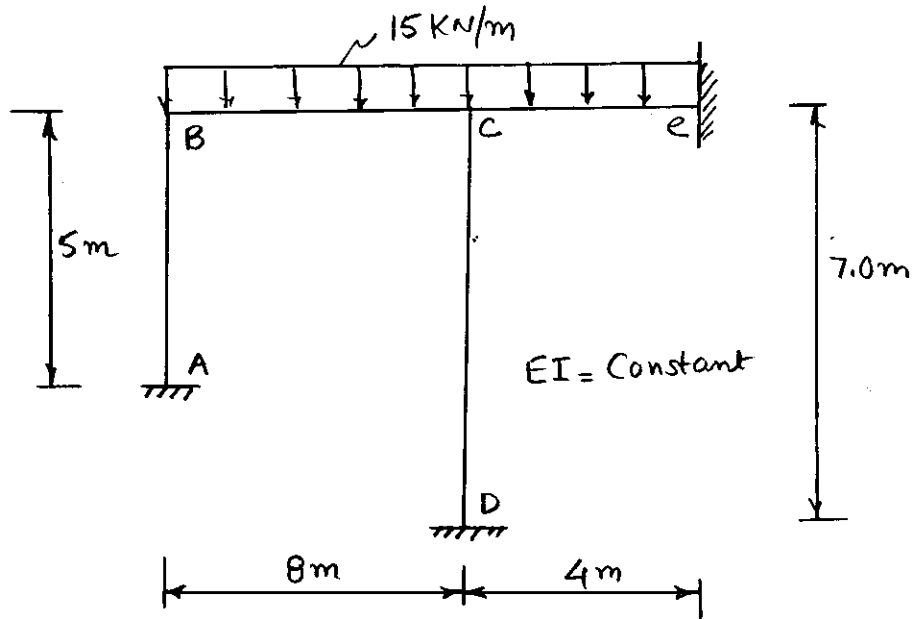


Advanced Structural Analysis

Midterm Exam - Solution

QNo1 Determine the moments in the portal Frame using moment Distribution method.



Stiffnesses & Relative Stiffnesses

$$K_{ab} = K_{ba} = \frac{I}{5} = 0.2K$$

$$K_{bc} = K_{cb} = \frac{I}{8} = 0.125K$$

$$K_{cd} = K_{dc} = \frac{I}{7} = 0.143K$$

$$K_{ce} = K_{ec} = \frac{I}{4} = 0.25K$$

Distribution Factors

$$D_{ba} = \frac{0.2}{0.2 + 0.125} = 0.615$$

$$D_{bc} = \frac{0.125}{0.2 + 0.125} = 0.385$$

$$D_{cb} = \frac{0.125}{0.125 + 0.25 + 0.143} = \frac{0.125}{0.518} = 0.241$$

$$D_{ce} = \frac{0.25}{0.518} = 0.483$$

$$D_{cd} = \frac{0.143}{0.518} = 0.276$$

Fixed End Moments

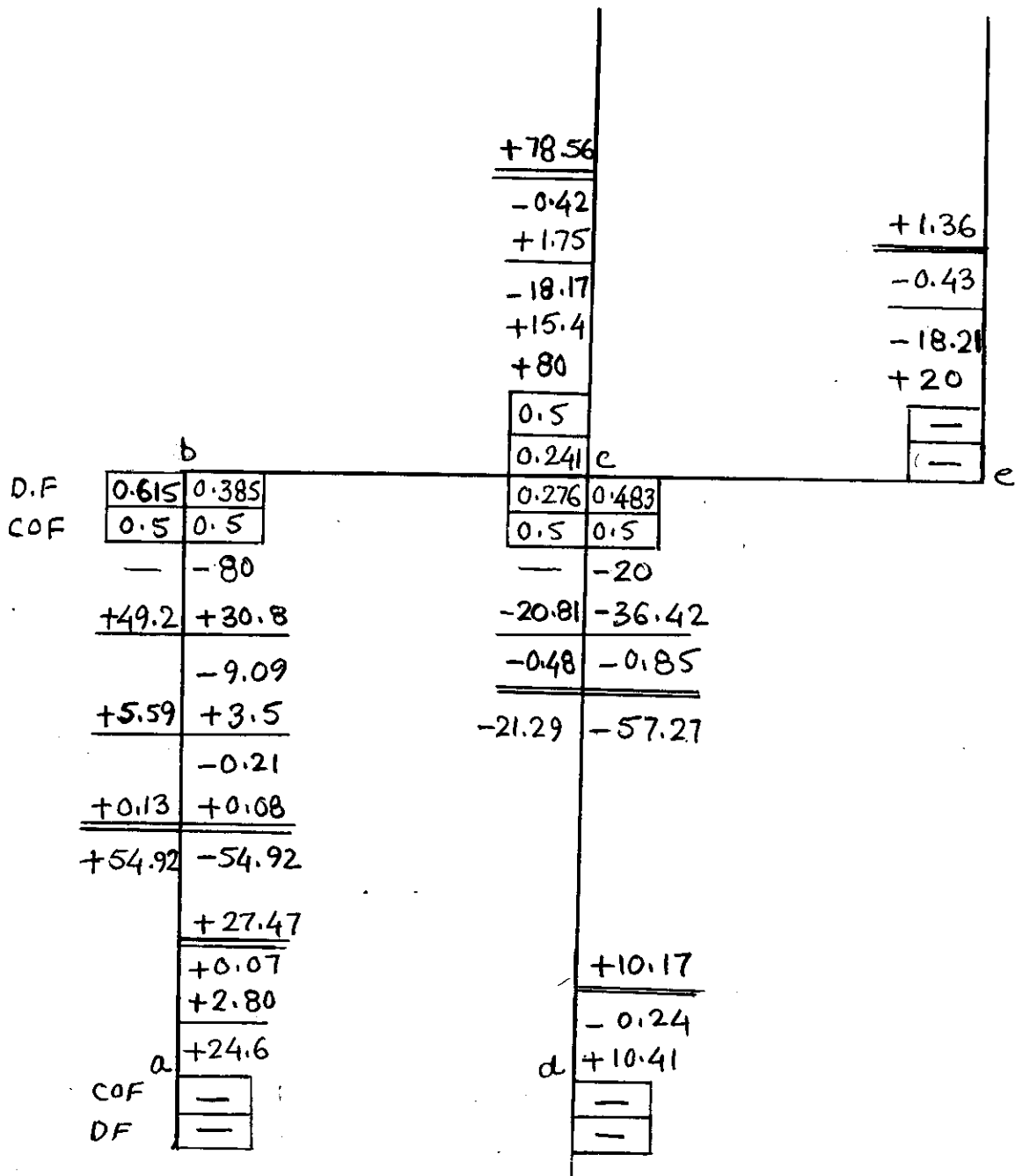
Clock-wise moment +ive

$$FEM_{bc} = -\frac{wl^2}{12} = \frac{15 \times 8^2}{12} = -80 \text{ KN-m}$$

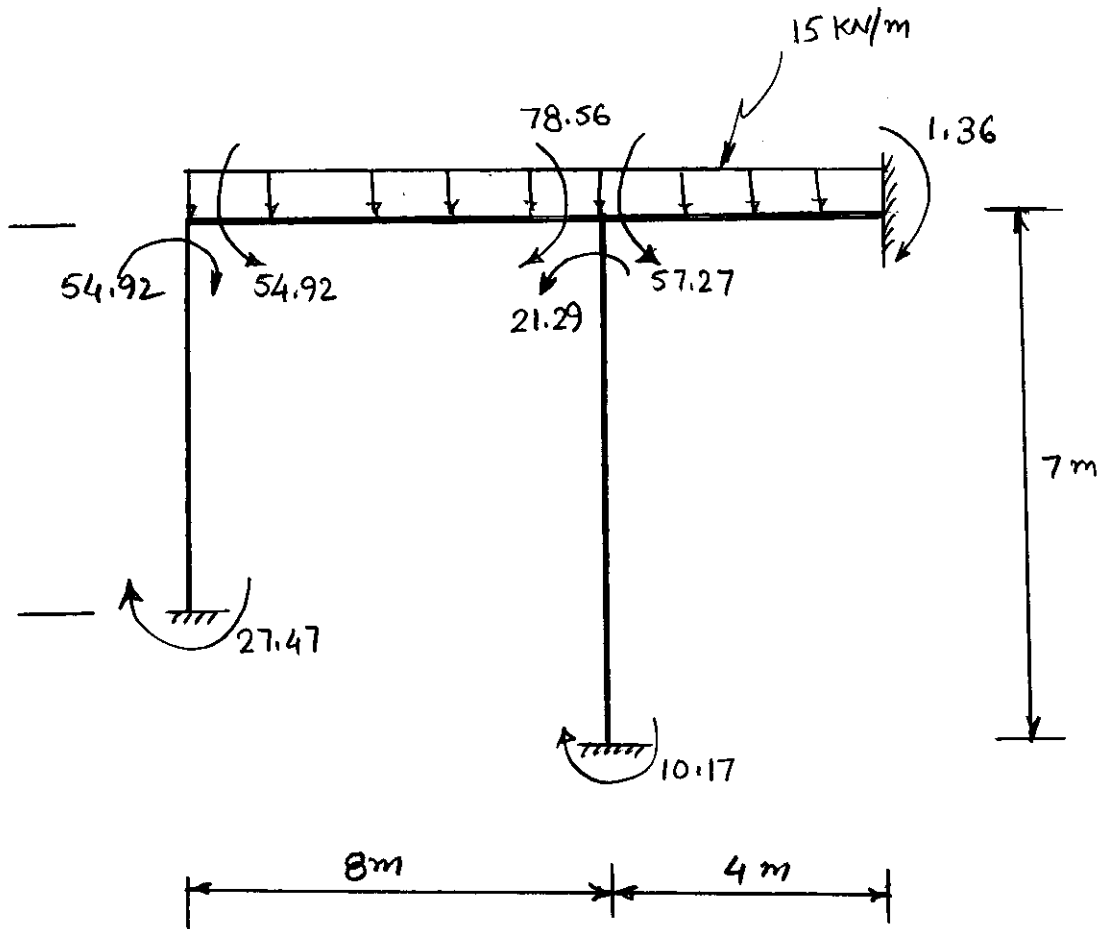
$$FEM_{cb} = +\frac{wl^2}{12} = +80 \text{ KN-m}$$

$$FEM_{ce} = -\frac{wl^2}{12} = \frac{15 \times 4^2}{12} = -20 \text{ KN-m}$$

$$FEM_{ec} = +\frac{wl^2}{12} = +20 \text{ KN-m}$$

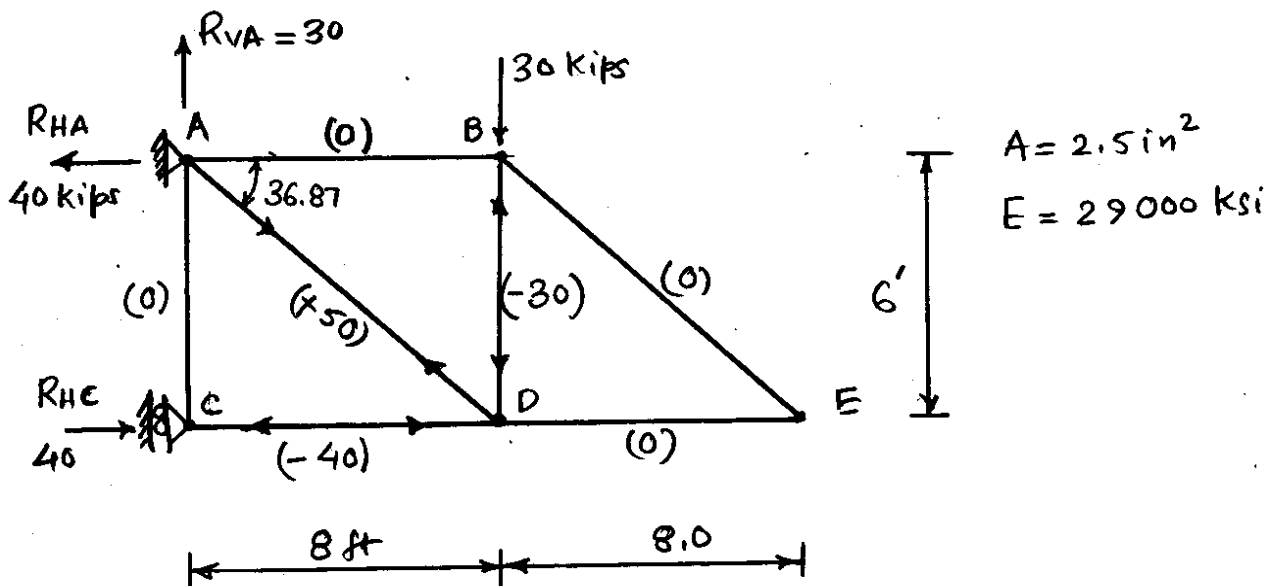


Mid term Exam



END MOMENTS

QNo.2 Find the vertical Deflection at pt E in the truss below using method of virtual work.



Determine Reactions

By observation $R_{VA} = 30 \text{ kips}$

Taking moments @ A

$$30 \times 8 - R_{HC} \times 6 = 0 \Rightarrow R_{HC} = \frac{30 \times 8}{6} = 40 \text{ kips}$$

$$\Rightarrow R_{HA} = -40 \text{ kips}$$

Member Forces

$$F_{CD} = -40 \text{ kips (Comp)}$$

By observation forces in members DE, BE = 0
as no nodal force @ E

$$F_{DE} = F_{BE} = 0$$

Also,

$F_{AC} = 0$ as no vertical reaction @ C

$$F_{AD} \sin 36.87 = R_{VA} = 30 \Rightarrow F_{AD} = \frac{30}{\sin 36.87} = +50 \text{ (Tens)}$$

No. 2

Consider joint D

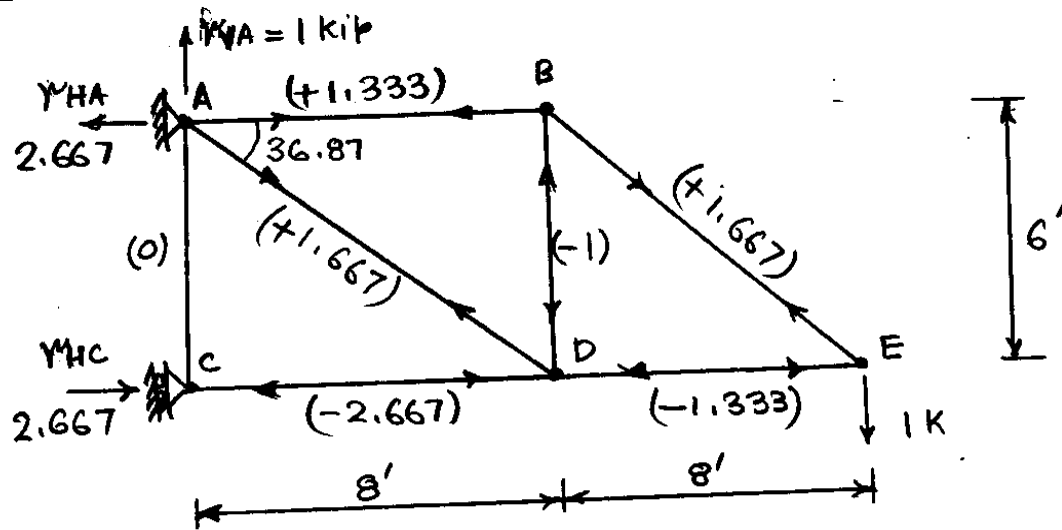
$$F_{AC} \sin 36.87 + F_{BD} = 0 \Rightarrow F_{BD} = -F_{AC} \sin 36.87$$

$$F_{BD} = -50 \sin 36.87$$

$$F_{BD} = -30 \text{ kip (Comp)}$$

Then The remaining Force F_{AB} has to be = 0 if equilibrium @ joint B is considered.

Forces in structure with Unit Load Applied



$$Y_{VA} = 1 \text{ kip } \uparrow$$

Moment @ A

$$1 \times 16 - Y_{HC} \times 6 = 0 \Rightarrow Y_{HC} = \frac{16}{6} = 2.667$$

$$\Rightarrow Y_{HA} = 2.667$$

$$F_{CD} = -2.667 \text{ kips (Comp)}$$

$$F_{AC}$$

$$F_{AD} \sin 36.87 = Y_{VA} = 1 \Rightarrow F_{AD} = \frac{1}{\sin 36.87} = +1.667 \text{ (Tens)}$$

@ Joint A

$$F_{AB} - Y_{HA} + F_{AD} \cos 36.87 = 0$$

$$\Rightarrow F_{AB} = 2.667 - 1.667 \cos 36.87 = +1.333 \text{ kips (Tens)}$$

QNo 2

At Joint E

$$f_{BE} \sin 36.87 - 1 = 0 \Rightarrow f_{BE} = \frac{1}{\sin 36.87} = +1.667 \text{ (Tens)}$$

$$f_{DE} + f_{BE} \cos 36.87 = 0 \Rightarrow f_{DE} = -1.667 \cos 36.87$$

$$f_{DE} = -1.333 \text{ Kips (Comp)}$$

@ Joint B

$$f_{BD} + f_{BE} \sin 36.87 = 0 \Rightarrow f_{BD} = -1.667 \sin 36.87$$

$$f_{BD} = -1.0 \text{ Kips (Comp)}$$

Apply Virtual Work Principle

$$1. \Delta_{VE} = \sum_{j=1}^m \frac{F \cdot f \cdot L}{AE}$$

F = Primary Structure Forces

f = Forces due to unit load.

Member	F (Kips)	L (ft)	f kip	FfL
AB	0	8	1.333	—
AC	0	6	0	—
AD	50	10	1.667	833.5
CD	-40	8	-2.667	853.44
BD	-30	6	-1.0	180.0
BE	0	10	+1.667	—
DE	0	8	-1.333	—
			Σ	1866.94

$$1. \Delta_{VE} = \sum_{j=1}^3 \frac{FfL}{AE} = \frac{1866.94}{\left(\frac{2.5 \text{ in}^2}{144}\right) (29000 \text{ ksi} \times 144)}$$

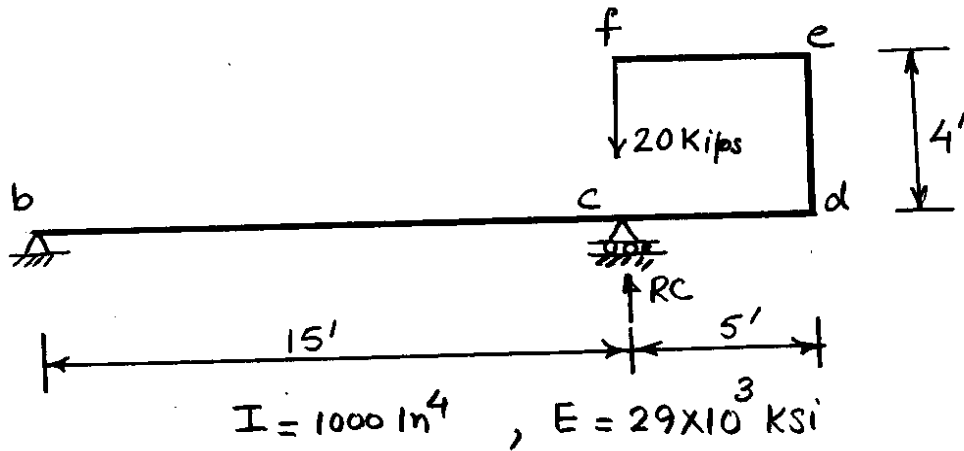
$$= \frac{1866.94}{2.5 \times 29000} = 0.02575 \text{ ft} \downarrow$$

$$= \underline{0.309 \sim 0.31 \text{ in} \downarrow}$$

Answer.

QNo 3

Calculate Vertical deflection at pt. f of the beam shown below using method of virtual work.



Reactions from Global Equilibrium

Taking moment @ pt b

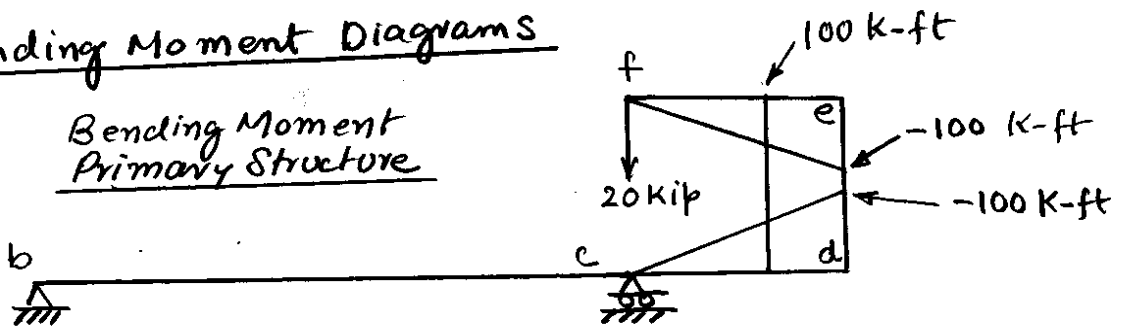
$$20 \times 15 - R_c \times 15 = 0$$

$$\Rightarrow R_c = 20 \text{ kips } \uparrow$$

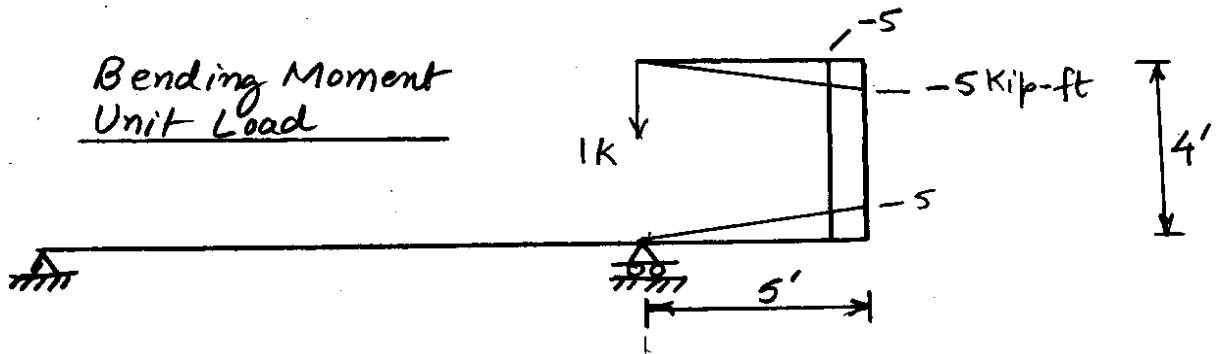
$$\Rightarrow R_b = 0$$

Bending Moment Diagrams

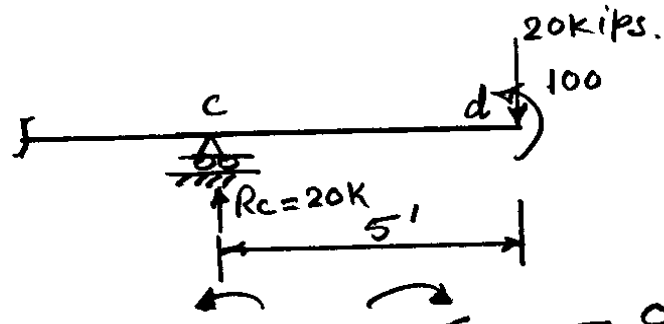
Bending Moment Primary Structure



Bending Moment Unit Load



Bending moment @ pt c



$$\text{Moment @ pt. c} = 100 - 20 \times 5 = 0$$

Virtual Work Expression

$$1. \Delta_{fv} = \sum_j \int_l \frac{Mm}{EI} dl$$

$$= \int_0^5 \frac{20x \cdot x}{EI} dx + \int_0^4 \frac{(-100)(-5)}{EI} dx + \int_0^5 \frac{20x \cdot x}{EI} dx$$

$$= 2 \int_0^5 \frac{20x^2}{EI} + \int_0^4 \frac{500}{EI} dx$$

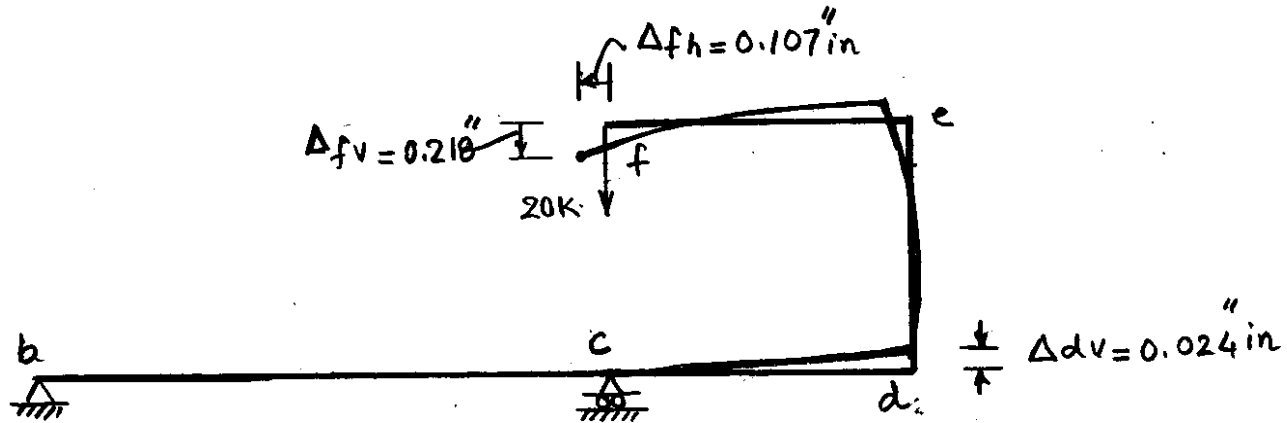
$$= \frac{1}{EI} \left\{ \left[\frac{40x^3}{3} \right]_0^5 + \left[500x \right]_0^4 \right\}$$

$$= \frac{1}{EI} \left\{ \frac{40}{3} \times 5^3 + 500 \times 4 \right\} = \frac{3666.67}{EI} \text{ kip-ft}$$

$$I = \frac{1000}{(12)^4} \text{ ft}^4, \quad E = 29000 \times 12^2 \text{ /ft}^2$$

$$1. \Delta_{fv} = \frac{3666.67 \times 12^2}{29000 \times 1000} = 0.0182 \text{ ft} = 0.218 \text{ in} \downarrow$$

QNo.3



Deformed Shape